Metl Span CFR-42 Roof System 24 Ga. Exterior / 26 Ga. Interior Facings Two or More Spans Conditions

CFR Panel	Design	LSD (Limit Design State), PSF					
Thickness	Criteria	Panel Span (ft)					
		2.5	3.0	4.0	5.0	6.0	7.0
42" wide	Bending & Shear	172.6	141.5	103.4	81.3	66.9	56.8
2" thick	Deflection (L/180)	160.4	131.1	94.1	71.7	56.5	45.7
2 fasteners/clip	Connection	82.0	74.9	65.6	59.6	48.3	40.0
42" wide	Bending & Shear	198.6	163.0	119.1	93.5	76.8	65.1
2.5" thick	Deflection (L/180)	196.7	161.4	116.8	89.8	71.7	58.6
2 fasteners/clip	Connection	90.9	81.9	70.4	63.1	51.2	42.5
42" wide	Bending & Shear	221.9	182.4	133.4	104.6	85.8	72.7
3" thick	Deflection (L/180)	228.7	188.2	137.1	106.1	85.2	70.2
2 fasteners/clip	Connection	99.9	88.9	75.0	66.6	54.2	45.0
42" wide	Bending & Shear	235.4	194.0	142.3	111.6	91.5	77.4
4" thick	Deflection (L/180)	280.0	231.2	169.7	132.5	107.5	89.5
2 fasteners/clip	Connection	117.7	102.7	83.9	72.5	59.4	50.0
42" wide	Bending & Shear	267.7	221.1	162.7	127.8	104.8	88.6
5" thick	Deflection (L/180)	313.9	259.8	191.8	150.6	123.0	103.1
3 fasteners/clip	Connection	121.0	105.7	86.5	74.8	62.1	53.1
42" wide	Bending & Shear	296.6	245.4	181.2	142.7	117.1	99.0
6" thick	Deflection (L/180)	330.4	273.9	203.0	160.1	131.3	110.6
3 fasteners/clip	Connection	124.3	108.7	89.0	77.0	64.8	56.0

Notes

- 1. Based on CFR-42 panel with 24 ga. exterior face (min $F_y = 50$ ksi) and 26 ga. interior face (min $F_y = 33$ ksi).
- Fastener pattern FPI: CFR panel clip and (2 or 3 as shown above) ¼"-14 Self-Drilling Tek 3 screws in min. 14 ga. steel or (2) ¼"-14 Self-Drilling Tek 3 screws in min. 12 ga. steel. Two fasteners per clip are required at end supports. In lieu of self-drilling screws, self-tapping screws may be used.
- 3. Factored resistance inward load is the lower of the panel bending and shear resistance.
- 4. Factored resistance outward load is the lowest value of panel bending, shear and connection resistances for each fastener pattern.
- 5. Connection loads may be increased with Fablok. Consult Metl-Span for additional loads.
- 6. The loads based on panel stress and deflection design criteria are derived from ASTM E-72 structural testing. The factored resistance loads are calculated with resistance factor of 0.5 and 0.4 for bending and shear stresses, respectively.
- 7. The panel and its connection strength was determined from ASTM E1592 testing and the factored resistance loads are calculated with a resistance factor of 0.7.
- 8. Specified loads should not exceed the deflection load for L/180 limit.
- 9. The structural capacity of the purlins are not considered and must be examined independently.
- 10. Multiple spans are based on 3 or more spans conditions.



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